



JdS 2021: Active learning strategy for fragility curve estimation using adaptive importance sampling

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– June 7, 2021 –

Introduction

- Seismic hazard
- Seismic probabilistic risk assesment
- Seismic fragility curve

Statistical learning of fragility curve

Adaptive importance sampling

- Motivation
- Principle
- Theoretical results

Numerical results

Conclusion

Introduction



Izmir seism picture (from newspaper *Le Dauphiné*, November 1st 2020)



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- Seisms have a huge social & human costs and are unfortunately random.
- Dramatic effect on mechanical structures.

Crucial need of seismic risk assesment for valuable industrial assets (e.g. nuclear power plant)



Nuclear power plant of Nogent sur Seine, France

A cost-benefit analysis between safety of crucial mechanical structures (e.g. nuclear power plant, dam) and uncertainties tainting seismic ground motion. It can be sumed up in 3 main steps ¹:

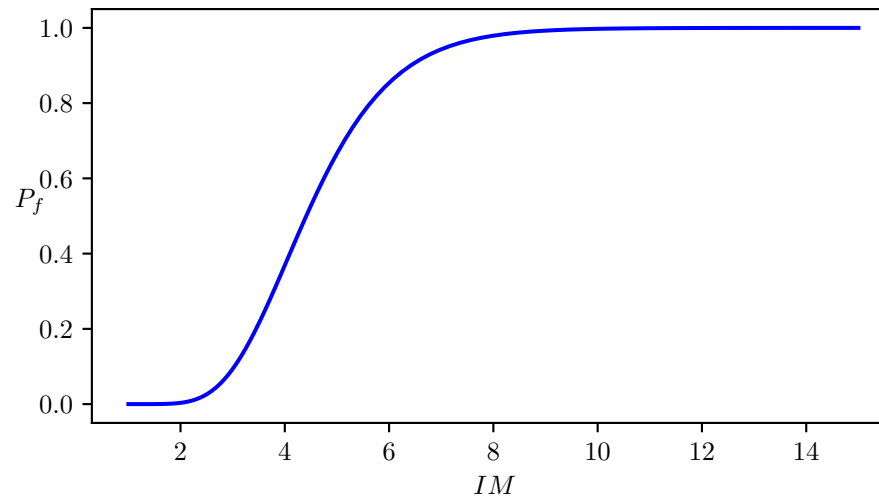
- 1) Estimation of the probability density function $p(a)$ of the intensity measure of a seism a .
- 2) **(my job)** Estimation of the conditional probability of failure given the seismic intensity a : the **fragility curve** $P_f(a)$
- 3) Estimation of a probability of failure:

$$\psi = \int P_f(a)p(a)da$$

¹Irmela Zentner, Max Gündel, and Nicolas Bonfils. Fragility analysis methods: Review of existing approaches and application. *Nuclear Engineering and Design*, 323:245 – 258, 2017.
ISSN 0029-5493.
doi: <https://doi.org/10.1016/j.nucengdes.2016.12.021>.
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$$P_f(a) = \mathbb{P}(L > C | IM = a)$$

- L : mechanical demand of the structure, obtained using time-consuming numerical simulations.
- C critical level where the structure is considered in a failure state.
- IM *Intensity Measure* of a seismic ground motions.



- Generate the data (X_i, S_i) where $S_i = \mathbf{1}_{L_i \geq C}$ and $X_i = \log(IM_i)$, using a computer model as an oracle. We use a synthetic generator of seismic signals to obtain X_i (e.g. a filtered modulated white noise).
- We suppose a lognormal shape for the fragility curve. This is a common parametric model used by mechanical engineers in seismic risk analysis:

$$\mathbb{P}(L > C | IM = a) = f_{\alpha, \beta}(a) = \Phi \left(\frac{\log \left(\frac{a}{\alpha} \right)}{\beta} \right)$$

- The parameters α and β are estimated by empirical risk minimization using least squares:

$$\hat{\alpha}_n, \hat{\beta}_n = \operatorname{argmin} \sum_{i=1}^n (S_i - f_{\alpha, \beta}(X_i))^2 \quad (1)$$

Adaptive importance sampling

- The labels $S_i = \mathbb{1}_{L_i \geq C}$ are gathered using time-consuming mechanical simulation (e.g. Finite Elements Method).

²Sanaz Rezaeian and Armen Der Kiureghian. Simulation of synthetic ground motions for specified earthquake and site characteristics. *Earthquake Engineering & Structural Dynamics*, 39(10):1155–1180, 2010.
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- The labels $S_i = \mathbb{1}_{L_i \geq C}$ are gathered using time-consuming mechanical simulation (e.g. Finite Elements Method).
- The log seismic intensity X_i are derived from synthetic ground motions ² which are considered cheap to generate.
- **Goal: Minimize the sample size and maximize the reduction of uncertainties concerning the fragility curve.** → **Active Learning**

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$$\hat{R}_n(\theta) = \frac{1}{n} \sum_{i=1}^n (S_i - f_\theta(X_i))^2$$

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■ IS-Empirical risk minimization:

$$\tilde{R}_n(\theta) = \frac{1}{n} \sum_{i=1}^n \frac{p(X_i)}{q(X_i)} (S_i - f_\theta(X_i))^2 .$$



$$\mathbb{V}(\tilde{R}_n(\theta)) = \frac{1}{n} \int \int \frac{p(x)^2}{q(x)} (s - f_\theta(x))^4 p(ds|x) dx - \mathbb{E}[(S - f_\theta(X))^2]^2$$


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$$q_\theta^* = \operatorname{argmin}_q \int_{x \in \mathcal{X}} \frac{p(x)^2}{q(x)} \tilde{\ell}_\theta(x) dx ,$$

$$\tilde{\ell}_\theta(x) = \mathbb{E}[(S - f_\theta(X))^4 | X = x]$$

$$q_\theta^*(x) \propto \sqrt{\tilde{\ell}_\theta(x)} p(x) ,$$

The optimal sampling density depends on θ ! \rightarrow *Adaptive* importance sampling

Because P_f is unknown we perform the following approximation of $\tilde{\ell}_\theta(x)$:

$$\tilde{\ell}_\theta(x) \approx f_\theta(x)(1 - f_\theta(x))^4 + (1 - f_\theta(x))f_\theta(x)^4$$

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Algorithm 3 Adaptive Importance Sampling

1. Initialisation: $\tilde{\theta}_0$
2. for $i = 1, \dots, n$:
 - 2.1 Sample X_i from $q_{\tilde{\theta}_{i-1}}$
 - 2.2 Call the mechanical simulation at point X_i to get label S_i
 - 2.3 Compute

$$\tilde{\theta}_i = \underset{\theta}{\operatorname{argmin}} \frac{1}{i} \sum_{j=1}^i \frac{p(X_j)}{q_{\tilde{\theta}_{j-1}}(X_j)} (S_j - f_\theta(X_j))^2$$

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■ *Defensive* AIS ³:

$$q_{\theta,\varepsilon}(x) = \varepsilon p(x) + (1 - \varepsilon)q_{\theta}^*(x)$$

■ *Penalized* least squares for stability and identifiability issues:

$$\Omega(\theta; \beta_{reg}) = \frac{\beta_{reg}}{\beta}$$

$$\tilde{R}_{n,reg}(\theta; \beta_{reg}) = \frac{1}{n} \sum_{i=1}^n \frac{p(X_i)}{q_{\tilde{\theta}_{i-1},\varepsilon}(X_i)} (S_i - f_{\theta}(X_i))^2 + \frac{\Omega(\theta; \beta_{reg})}{n}$$

$$\tilde{\theta}_{n,\varepsilon} = \operatorname{argmin} \tilde{R}_{n,reg}(\theta; \beta_{reg})$$

³Art Owen and Yi Zhou. *Safe and effective importance sampling*.

Journal of the American Statistical Association, 95(449):135–143, 2000.

ISSN 01621459.

URL <http://www.jstor.org/stable/2669533>

Theoretical results

Theorem (Asymptotic normality of $\tilde{\theta}_n$)

Denote $\theta_* = \operatorname{argmin} \mathbb{E}[(S - f_\theta(X))^2]$. Suppose $\tilde{\theta}_n$ consistent. With appropriate regularity conditions, $\sqrt{n}(\tilde{\theta}_n - \theta_*)$ is asymptotically normal with mean zero and covariance matrix $\ddot{R}(\theta_*)^{-1}V(q_{\theta_*,\varepsilon})(\ddot{R}(\theta_*)^{-1})^T$ with

$$V(q_{\theta_*,\varepsilon}) = \mathbb{E} \left[\frac{p(X)}{q_{\theta_*,\varepsilon}(X)} (S - f_{\theta_*}(X))^2 \nabla f_{\theta_*}(X) \nabla f_{\theta_*}(X)^T \right]$$

■ Tools: M-estimation theory ⁴ and convergence of martingale ⁵.

⁴A. W. van der Vaart. *Asymptotic Statistics*. Cambridge Series in Statistical and Probabilistic Mathematics. Cambridge University Press, 1998.
doi: [10.1017/CBO9780511802256](https://doi.org/10.1017/CBO9780511802256)

⁵P. Hall, C.C. Heyde, Z.W. Birnbaum, and E. Lukacs. *Martingale Limit Theory and Its Application*. Communication and Behavior. Elsevier Science, 2014.
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- For the lognormal parametric model $f_{\alpha,\beta}(a) = \Phi\left(\frac{\log(\frac{a}{\alpha})}{\beta}\right)$. The theorem is valid as long as $(\alpha, \beta) \in \Theta$ where Θ is a compact in \mathbb{R}^2 .
- Usually $\Theta = [\alpha_{min}, \alpha_{max}] \times [\beta_{min}, \beta_{max}]$

- Question: at which sample size does the parameter $\tilde{\theta}_n$ reaches asymptotic normality ?

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- Consider two independent datasets $\mathcal{D}_1 = (X_{i,1}, S_{i,1})_{i \in \{1, \dots, n\}}$ and $\mathcal{D}_2 = (X_{i,2}, S_{i,2})_{i \in \{1, \dots, n\}}$.

$$\tilde{\theta}_{n,j} = \operatorname{argmin}_{\theta \in \Theta} \tilde{R}_{n,j}(\theta), \quad j = 1, 2.$$

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$$\sqrt{n}(\dot{\tilde{R}}_{n,1}(\tilde{\theta}_{n,2}) - \dot{\tilde{R}}_{n,2}(\tilde{\theta}_{n,1})) \xrightarrow{\mathcal{L}} \mathcal{N}(0, 8V(q_{\theta_*, \varepsilon}))$$

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- Stopping criterion:

$$\widehat{W}_n = \frac{n}{8} (\dot{\tilde{R}}_{n,1}(\tilde{\theta}_{n,2}) - \dot{\tilde{R}}_{n,2}(\tilde{\theta}_{n,1}))^T \widehat{V}_{n,12}^{-1} (\dot{\tilde{R}}_{n,1}(\tilde{\theta}_{n,2}) - \dot{\tilde{R}}_{n,2}(\tilde{\theta}_{n,1}))$$

$$\widehat{V}_{n,12} = \frac{1}{2} (\widehat{V}_{n,1}(\tilde{\theta}_{n,1}) + \widehat{V}_{n,2}(\tilde{\theta}_{n,2}))$$

$$\widehat{V}_n(\theta) = \frac{1}{n} \sum_{i=1}^n \frac{p(X_i)^2}{q_{\theta, \varepsilon}(X_i) q_{\tilde{\theta}_{i-1}, \varepsilon}(X_i)} (S_i - f_\theta(X_i))^2 \nabla f_\theta(X_i) \nabla f_\theta(X_i)^T$$

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- By Slutsky's lemma, \widehat{W}_n converges weakly to $\chi^2(2)$.

Numerical results

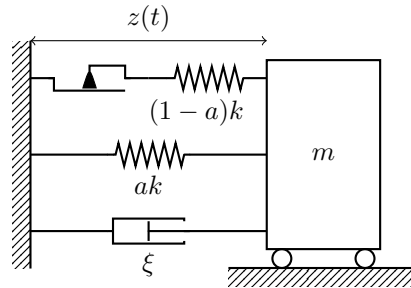
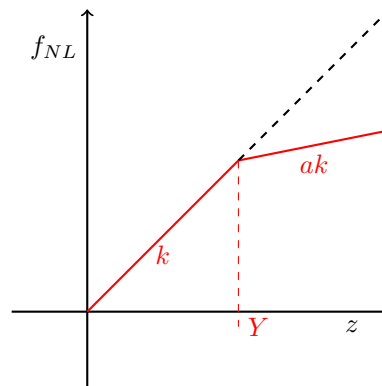


Figure: Elasto-plastic mechanical oscillator with kinematic hardening with parameter, $f_L = 5$ Hz, $\beta = 2\%$, the yield limit is $Y = 5.10^{-3}$ m and the post-yield stiffness is 20% of the elastic stiffness hence $a = 0.2$



$$\ddot{z}(t) + 2\beta\omega_L\dot{z}(t) + f_{NL}(t) = -s(t) ,$$

- Consider a dataset of 10^5 sythetic seismic signal $t \rightarrow s_i(t)$.
- $z_i(t)$ displacement of the oscillator corresponding to signal s_i .
- A natural mechanical demand:

$$L_i = \max_{t \in [0, T]} |z_i(t)| .$$

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- A natural mechanical demand:

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- Failure state for this oscillator:

$$S_i = \mathbb{1}_{(L_i > C)}$$

- $C = 2Y$ approximately a 90% quantile of the mechanical response L .
- Our seismic intensity measure will be the peak ground acceleration (PGA):

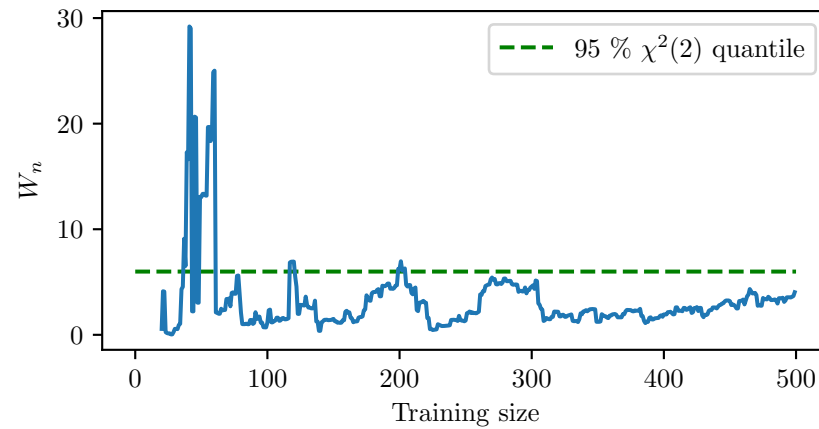
$$PGA_i = \max_{t \in [0, T]} |s_i(t)|$$

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- 500 replications of 100 queries in the unlabeled pool of 10^5 signals using acceptance reject algorithm.



Value of the stopping criterion for two independent samples with varying sizes from 20 to 500.

The observed values of \widehat{W}_n suggests to stop at $n = 100$ and then to build asymptotic confidence interval.

Comparison between AIS and sampling at random.

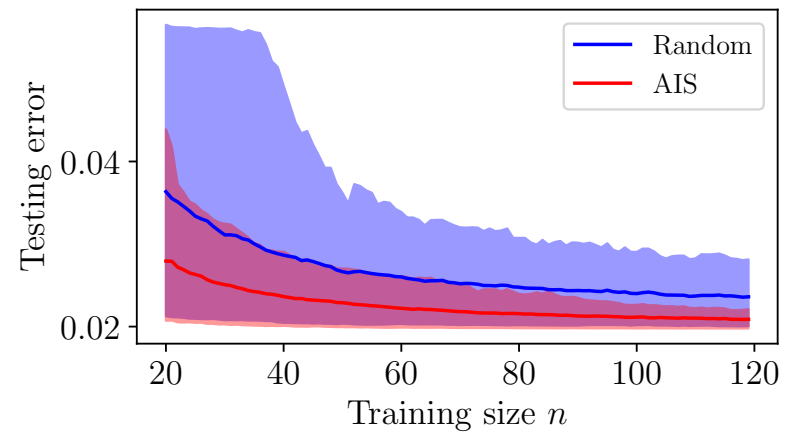
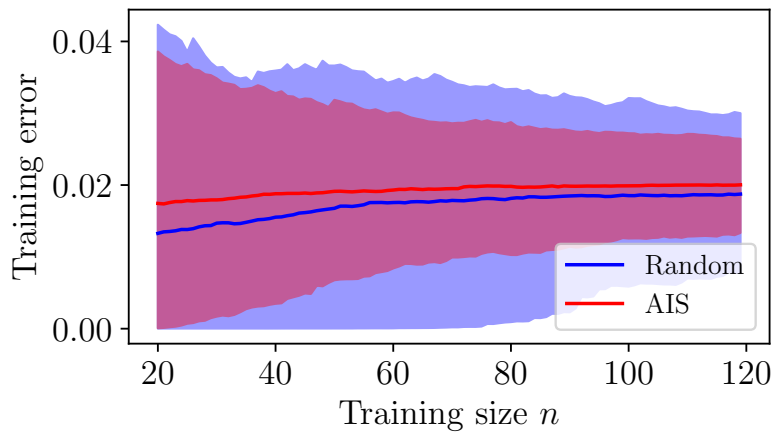
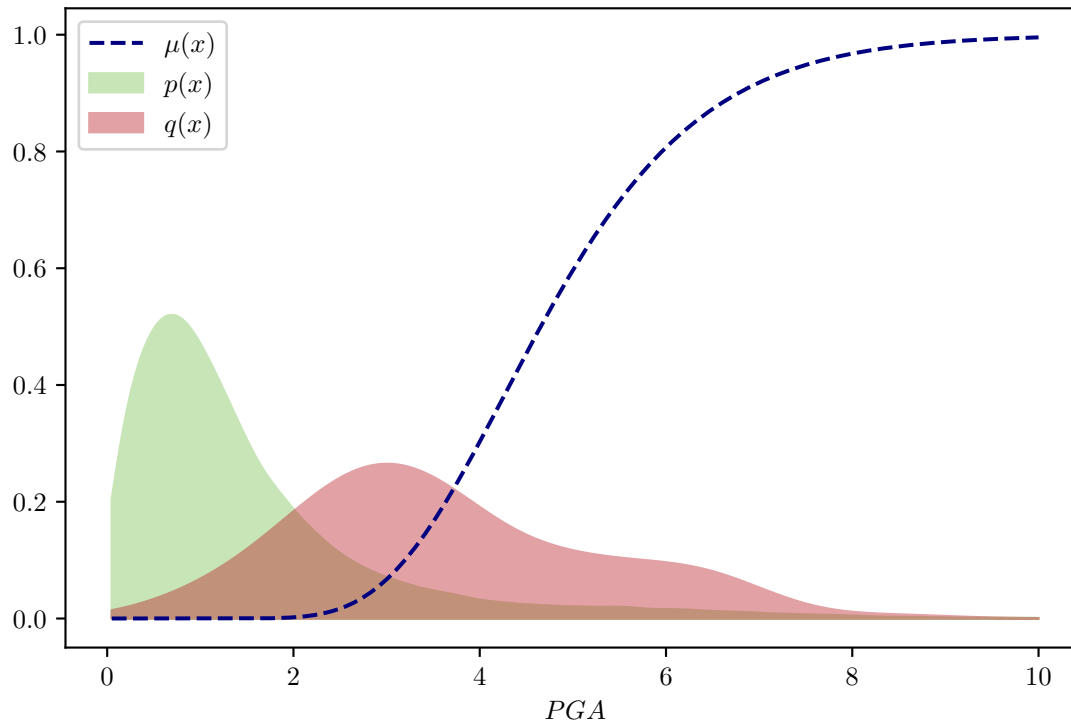


Figure: Numerical benchmark of the nonlinear oscillator.



Estimation of the marginal density of PGA compared to the estimation of the density of sampled PGA using AIS with 100 datapoints for the nonlinear oscillator.

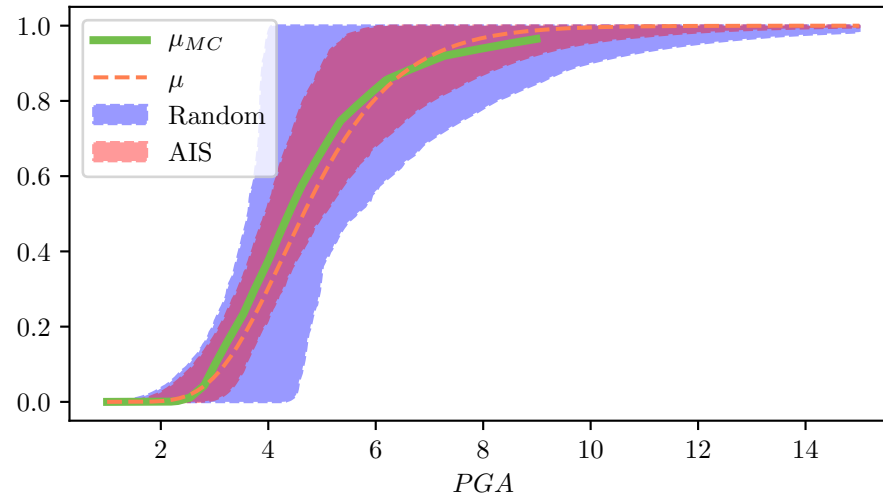


Figure: Estimation of the confidence intervals for random sampling and ADIS for the nonlinear oscillator

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- Paper soon to be submitted in *Reliability assessment and safety engineering*.

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- Paper soon to be submitted in *Reliability assessment and safety engineering*.
- **Program for the rest of the thesis:** Uncertainty propagation of the mechanical parameters of the structure, construction of a multi-fidelity metamodel.

Thank you for your attention !

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Merci pour votre attention !

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